

**STRUCTURAL
CALCULATIONS**

Nestler-Spare Residence
8265 SE 61st St.
Mercer Island, WA 98040

Studio Diaa
3125 Eastlake Ave E, Suite C
Seattle, WA 98102

August 8, 2025



NESTLER - SPARE

- POST - PERMIT REVISION (#3)

- WALL SHIFTS IN NW CORNER

- REVISED FRAMING CALCULATIONS ATTACHED
FOR CHANGED MEMBERS

- ADDED PIN PILES TO FOUNDATION PER GEOTECH

CQN

CARTER QUINN NORLIN

**STRUCTURAL
ENGINEERING**

► 2033 Sixth Avenue #995 Seattle, WA 98121 206-264-7784 www.CQN-SE.com

PROJECT:

NESTLER - SPARE

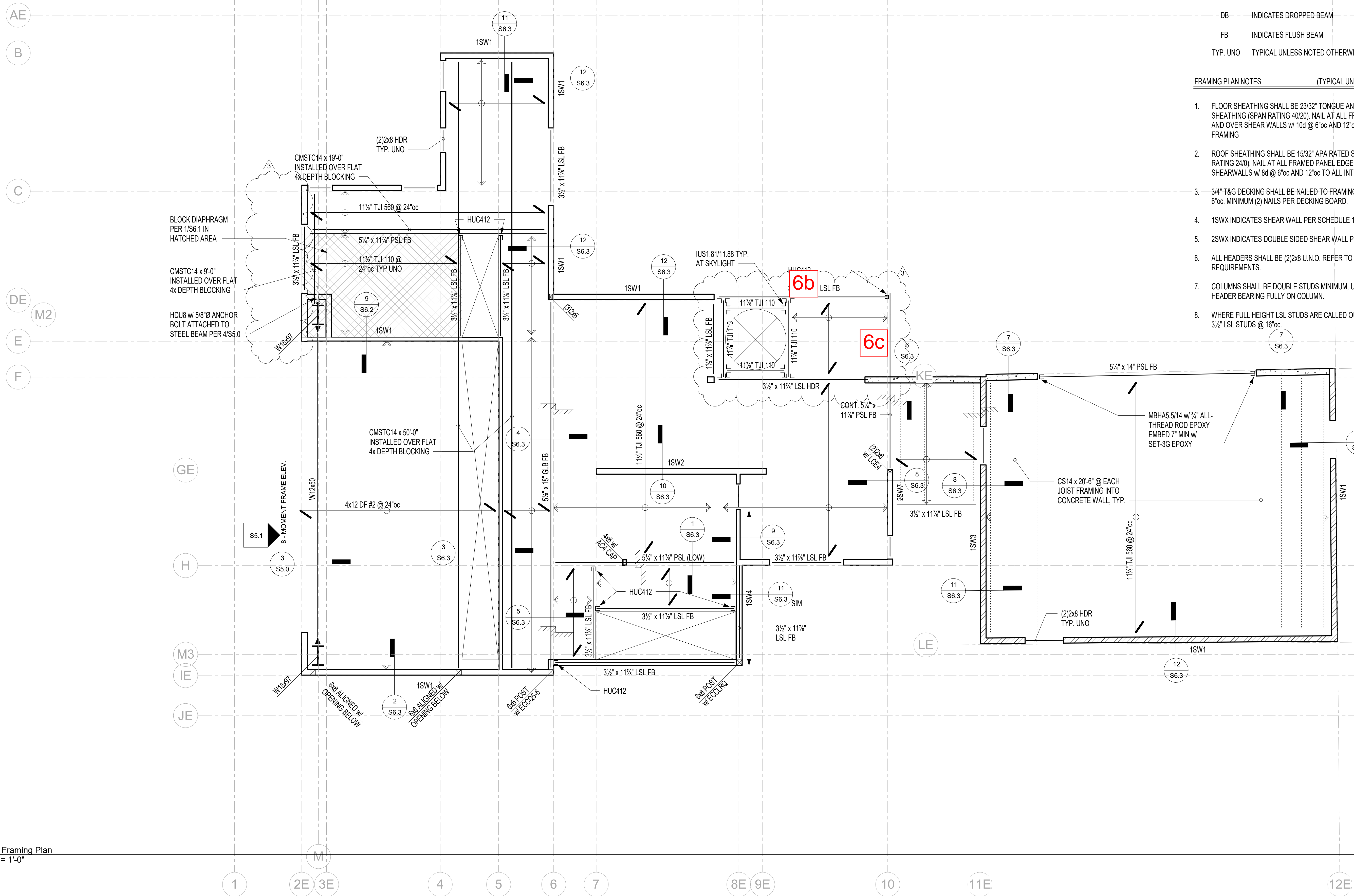
DATE:

07/28/25

DESIGNER:

SN

SHEET #:



FRAMING PLAN LEGEND

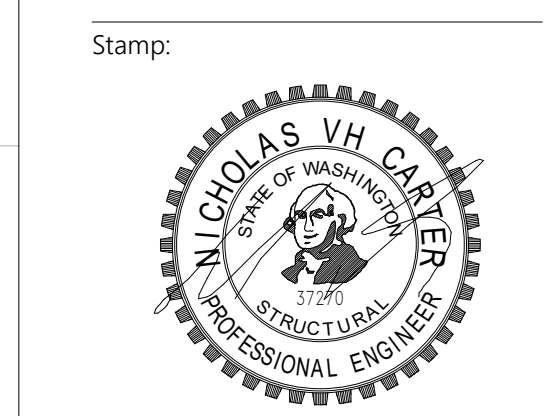
- WALLS BELOW
- COLUMNS BELOW
- HANGER
- ABRUPT CHANGE IN SLAB/FRAMING ELEVATION
- INDICATES DETAIL X ON SHEET SX.XX
- FRAMING SPAN AND EXTENTS
- DB INDICATES DROPPED BEAM
- FB INDICATES FLUSH BEAM
- TYP. UNO TYPICAL UNLESS NOTED OTHERWISE

FRAMING PLAN NOTES (TYPICAL UNLESS NOTED OTHERWISE)

1. FLOOR SHEATHING SHALL BE 23/32" TONGUE AND GROOVE APA RATED SHEATHING (SPAN RATING 40/20). NAIL AT ALL FRAMED PANEL EDGES AND OVER SHEAR WALLS w/ 10d @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING
2. ROOF SHEATHING SHALL BE 15/32" APA RATED SHEATHING (SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER SHEARWALLS w/ 8d @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING.
3. 3/4" T&G DECKING SHALL BE NAILED TO FRAMING BELOW WITH 10d @ 6"oc. MINIMUM (2) NAILS PER DECKING BOARD.
4. 1SWX INDICATES SHEAR WALL PER SCHEDULE 12/S6.0.
5. 2SWX INDICATES DOUBLE SIDED SHEAR WALL PER SCHEDULE 12/S6.0.
6. ALL HEADERS SHALL BE (2)2x8 U.N.O. REFER TO NOTE 5 FOR SUPPORT REQUIREMENTS.
7. COLUMNS SHALL BE DOUBLE STUDS MINIMUM. U.N.O., WITH BEAM OR HEADER BEARING FULLY ON COLUMN.
8. WHERE FULL HEIGHT LSL STUDS ARE CALLED OUT, INSTALL 1.3E 1 1/2" x 3/2" LSL STUDS @ 16"oc.

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Revision: Date:
 MERCER ISLAND 03.28.25
 BUILDING PERMIT REV 1
 MERCER ISLAND 06.10.25
 BUILDING PERMIT REV 2



Consultants:

Project:
NS Residence
 project no. 2401
 8265 SE 61st St
 Mercer Island, WA 98040

Drawing Title:
Roof Framing Plan

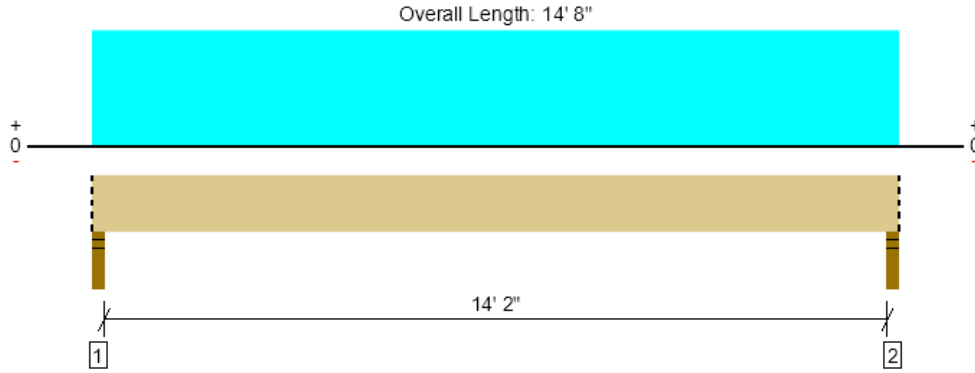
Date: June 9, 2025
 Issued For: Permit Comments #2
 Drawn By:
 Checked By:
 Scale: As indicated
 Sheet No.:

S2.3

1 Roof Framing Plan
 1/4" = 1'-0"

Roof Framing, Beam 6b

1 piece(s) 3 1/2" x 11 7/8" 1.55E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1250 @ 1 1/2"	4253 (3.00")	Passed (29%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	1039 @ 1' 2 7/8"	9878	Passed (11%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	4429 @ 7' 4"	18346	Passed (24%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.120 @ 7' 4"	0.481	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.235 @ 7' 4"	0.721	Passed (L/737)	--	1.0 D + 1.0 S (All Spans)

Member Length : 14' 8"
 System : Roof
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD
 Member Pitch : 0/12

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Stud wall - HF	3.00"	3.00"	1.50"	609	642	1250	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.50"	609	642	1250	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	14' 8" o/c	
Bottom Edge (Lu)	14' 8" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 14' 8"	N/A	13.0	--	
1 - Uniform (PSF)	0 to 14' 8" (Front)	3' 6"	20.0	25.0	Default Load

• Side loads are assumed to not induce cross-grain tension.

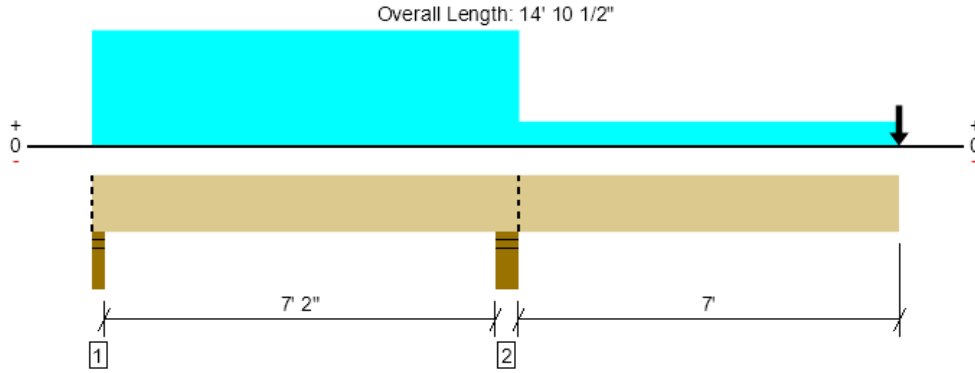
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 The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



Roof Framing, Beam 6c

1 piece(s) 5 1/4" x 11 7/8" 2.0E Parallam® PSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4060 @ 7' 7 3/4"	11694 (5.50")	Passed (35%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2020 @ 6' 5 1/8"	13861	Passed (15%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	-10733 @ 7' 7 3/4"	34332	Passed (31%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.219 @ 14' 10 1/2"	0.482	Passed (2L/790)	--	1.0 D + 1.0 S (Alt Spans)
Total Load Defl. (in)	0.435 @ 14' 10 1/2"	0.723	Passed (2L/400)	--	1.0 D + 1.0 S (Alt Spans)

Member Length : 14' 10 1/2"
 System : Roof
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD
 Member Pitch : 0/12

- Deflection criteria: LL (L/360) and TL (L/240).
- Overhang deflection criteria: LL (2L/360) and TL (2L/240).
- Right cantilever length exceeds 1/3 member length or 1/2 back span length. Additional bracing should be considered.
- Allowed moment does not reflect the adjustment for the beam stability factor.
- -752 lbs uplift at support located at 1 1/2". Strapping or other restraint may be required.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Stud wall - HF	3.00"	3.00"	1.50"	-278	109/-474	-752	Blocking
2 - Stud wall - HF	5.50"	5.50"	1.91"	2065	1995	4060	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	14' 11" o/c	
Bottom Edge (Lu)	14' 11" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 14' 10 1/2"	N/A	19.5	--	
1 - Uniform (PSF)	0 to 14' 10 1/2" (Front)	1'	20.0	25.0	Default Load
2 - Uniform (PSF)	0 to 7' 10 1/2" (Front)	3' 9"	20.0	25.0	Default Load
3 - Point (lb)	14' 10 1/2" (Front)	N/A	609	642	Linked from: Beam 6b, Support 2

• Side loads are assumed to not induce cross-grain tension.

Weyerhaeuser Notes

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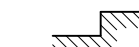
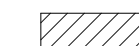

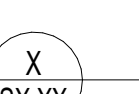



The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



7/28/2025 8:06:16 PM UTC
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FOUNDATION PLAN LEGEND

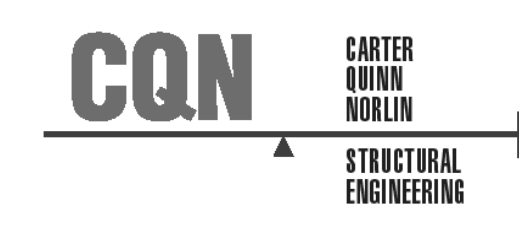
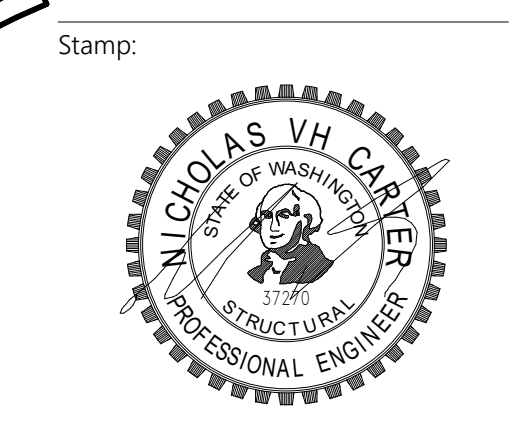
-  ABRUPT CHANGE IN SLAB/FRAMING ELEVATION
-  INDICATES EXISTING FOUNDATION
-  INDICATES NEW FOUNDATION
-  INDICATES DETAIL X ON SHEET SX.XX
-  INDICATES SIMPSON HOLDOWN. REFER TO DETAIL 10/S3.0 & FOUNDATION PLAN NOTES FOR ANCHOR AND STUD STACK REQUIREMENTS.
-  2"Ø PIPE PILE. REFER TO DETAILS ON SHEETS S3.0 & S3.1 FOR BATTERED PILES, WHERE APPLICABLE
-  EPOXY EMBED (2)#4x2'-0" BOT INTO (E) FOUNDATION 4" MIN USING SET-3G EPOXY

FOUNDATION PLAN NOTES

1. SLABS ON GRADE SHALL BE 4" THICK WITH 6x6 W1.4xW1.4 WWM CENTERED, U.N.O. PREPARED SOILS AND PROVIDE MINIMUM 10-MIL VISQUEEN VAPOR BARRIER UNDER ALL SLABS. REFER TO ARCHITECTURAL PLANS FOR SLAB ON GRADE ELEVATION AND TOPPING SLAB, WHERE OCCURS.
2. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS AND TOP OF SLAB ELEVATIONS.
3. ALL HOLDOWNS TO BE INSTALLED PER REQUIRED BY MANUFACTURER'S HOLDOWN SCHEDULE 10/S3.0. WALL PER ARCH
4. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS AND NOTIFY ENGINEER OF ANY DISCREPANCIES.

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Revision: Date:
MERCER ISLAND BUILDING PERMIT REV 1 03.28.25
MERCER ISLAND BUILDING PERMIT REV 2 06.10.25



Consultants:

Project:
NS Residence
project no. 2401
8265 SE 61st St
Mercer Island, WA 98040

Drawing Title:
Upper Floor Framing Plan

Date: June 9, 2025

Issued For: Permit Comments #2

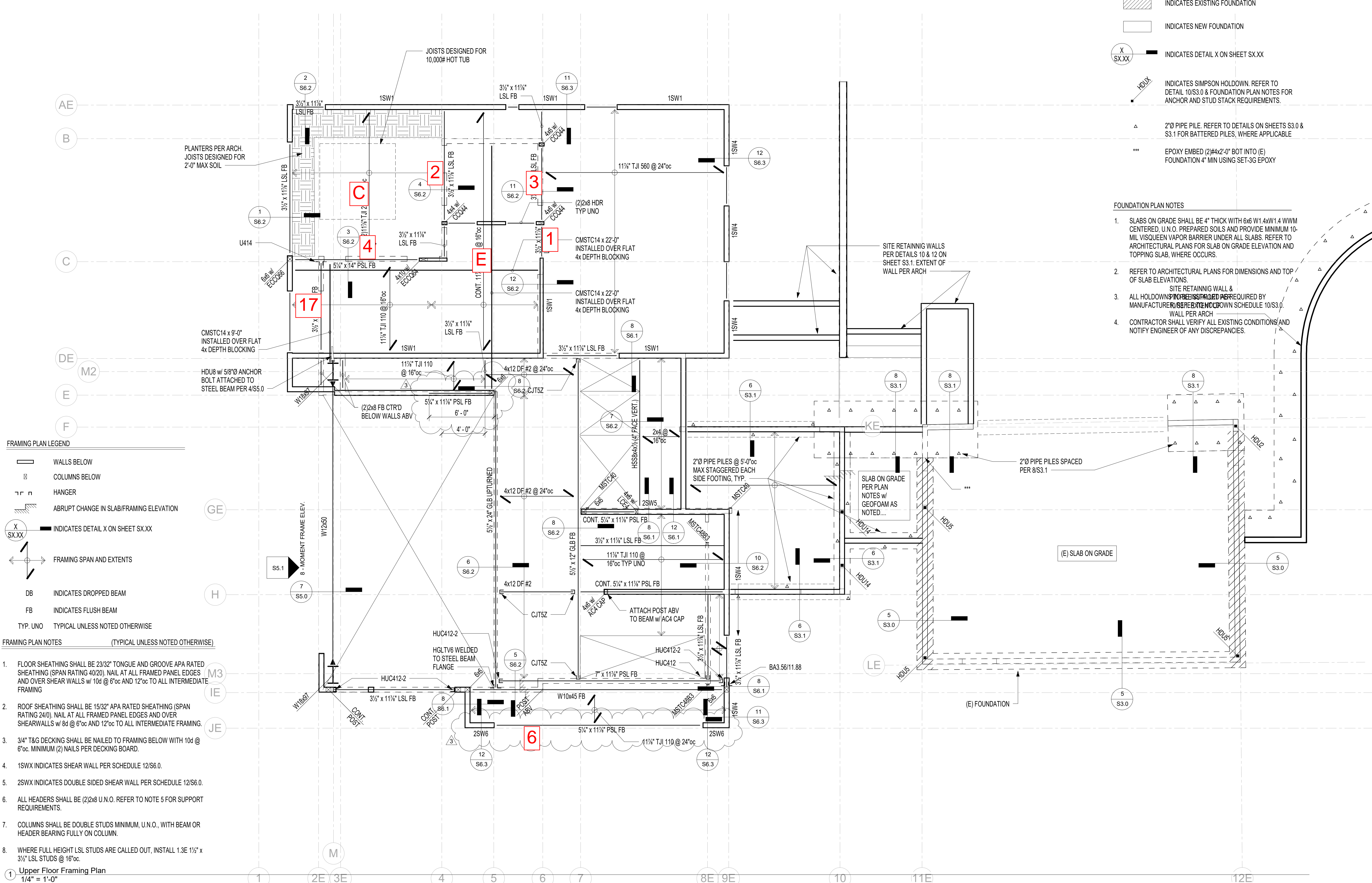
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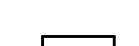



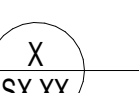




Scale: As indicated

Sheet No.:

S2.2



FRAMING PLAN LEGEND

-  WALLS BELOW
-  COLUMNS BELOW
-  HANGER
-  ABRUPT CHANGE IN SLAB/FRAMING ELEVATION
-  INDICATES DETAIL X ON SHEET SX.XX
-  FRAMING SPAN AND EXTENTS
-  DB INDICATES DROPPED BEAM
-  FB INDICATES FLUSH BEAM
-  TYP. UNO TYPICAL UNLESS NOTED OTHERWISE

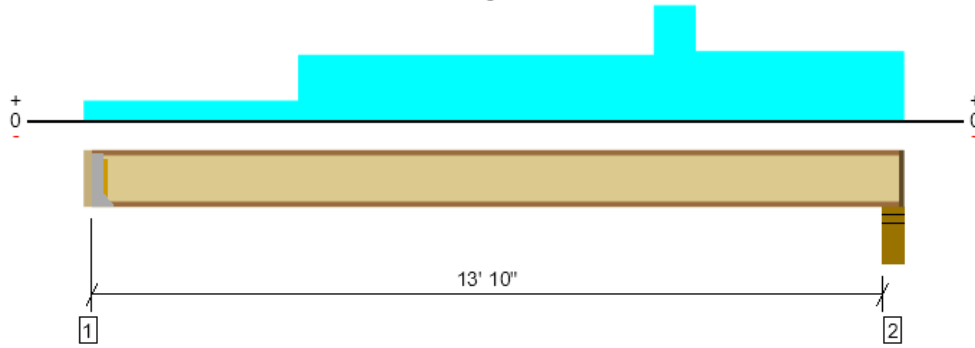
FRAMING PLAN NOTES (TYPICAL UNLESS NOTED OTHERWISE)

1. FLOOR SHEATHING SHALL BE 23/32" TONGUE AND GROOVE APA RATED SHEATHING (SPAN RATING 40/20). NAIL AT ALL FRAMED PANEL EDGES AND OVER SHEAR WALLS w/ 10d @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING
2. ROOF SHEATHING SHALL BE 15/32" APA RATED SHEATHING (SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER SHEAR WALLS w/ 8d @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING.
3. 3/4" T&G DECKING SHALL BE NAILED TO FRAMING BELOW WITH 10d @ 6"oc. MINIMUM (2) NAILS PER DECKING BOARD.
4. 1SWX INDICATES SHEAR WALL PER SCHEDULE 12/S6.0.
5. 2SWX INDICATES DOUBLE SIDED SHEAR WALL PER SCHEDULE 12/S6.0.
6. ALL HEADERS SHALL BE (2)2x8 U.N.O. REFER TO NOTE 5 FOR SUPPORT REQUIREMENTS.
7. COLUMNS SHALL BE DOUBLE STUDS MINIMUM, U.N.O., WITH BEAM OR HEADER BEARING FULLY ON COLUMN.
8. WHERE FULL HEIGHT LSL STUDS ARE CALLED OUT, INSTALL 1.3E 1 1/2" x 3 1/2" LSL STUDS @ 16"oc.

1 Upper Floor Framing Plan
1/4" = 1'-0"

Upper Floor Framing, Joist C
2 piece(s) 11 7/8" TJI® 210 @ 12" OC

Overall Length: 14' 5 1/2"



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2207 @ 14' 1"	2920 (3.50")	Passed (76%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	2097 @ 14'	3310	Passed (63%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	6867 @ 7' 7 3/8"	7590	Passed (90%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.296 @ 7' 1 11/16"	0.348	Passed (L/563)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.400 @ 7' 3 3/16"	0.696	Passed (L/418)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	62	45	Passed	--	--

Member Length : 14' 2 1/4"
 System : Floor
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: None.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories	Details
	Total	Available	Required	Dead	Floor Live	Factored		
1 - Hanger on 11 7/8" HF beam	2.00"	Hanger ¹	1.75" / - ²	346	1132	1478	See note ¹	W
2 - Stud wall - HF	5.50"	4.25"	2.13"	1069	1170	2239	1 1/4" Rim Board	A3

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.
- ² Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 10" o/c	
Bottom Edge (Lu)	14' 2" o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	MIU4.28/9	2.50"	N/A	16-10d	2-10dx1.5	Web Stiffeners

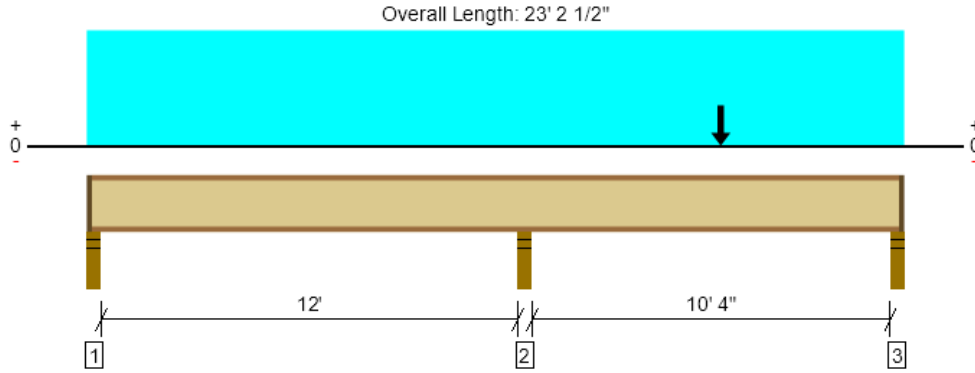
- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 14' 5 1/2"	12"	30.0	60.0	Default Load
2 - Uniform (PSF)	10' to 14' 5 1/2"	12"	220.0	--	Default Load
3 - Uniform (PSF)	3' 9" to 10' 9"	12"	--	205.0	Default Load

ForteWEB Software Operator	Job Notes
Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



Upper Floor Framing, Joist E
1 piece(s) 11 7/8" TJI@ 110 @ 16" OC



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1133 @ 12' 5 1/4"	1935 (3.50")	Passed (59%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	539 @ 12' 7"	1716	Passed (31%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	-1329 @ 12' 5 1/4"	3160	Passed (42%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.072 @ 5' 11 5/8"	0.306	Passed (L/999+)	--	1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.088 @ 5' 9 3/8"	0.611	Passed (L/999+)	--	1.0 D + 1.0 L (Alt Spans)
TJ-Pro™ Rating	57	45	Passed	--	--

Member Length : 23'
 System : Floor
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: None.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories	Details
	Total	Available	Required	Dead	Floor Live	Factored		
1 - Stud wall - HF	3.50"	2.25"	1.75"	90	293/-23	383	1 1/4" Rim Board	A3
2 - Stud wall - SPF	3.50"	3.50"	3.50"	370	763	1133	None	--
3 - Stud wall - HF	3.50"	2.25"	1.75"	137	260/-45	398	1 1/4" Rim Board	A3

• Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	5' 9" o/c	
Bottom Edge (Lu)	4' 11" o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 23' 2 1/2"	16"	15.0	40.0	Default Load
2 - Point (PLF)	18'	16"	100.0	--	

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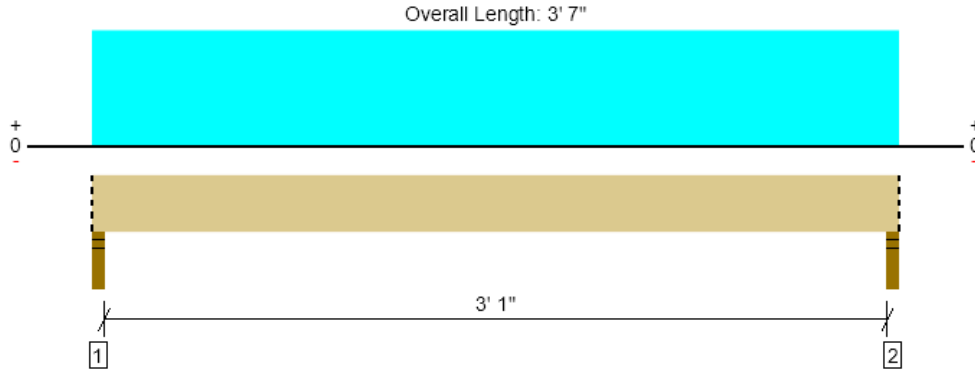
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Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



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 File Name: Nestler-Spare (2025)

Upper Floor Framing, Beam 1

1 piece(s) 3 1/2" x 11 7/8" 1.55E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1573 @ 1 1/2"	4253 (3.00")	Passed (37%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	485 @ 1' 2 7/8"	9878	Passed (5%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	1219 @ 1' 9 1/2"	18346	Passed (7%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.002 @ 1' 9 1/2"	0.083	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.008 @ 1' 9 1/2"	0.167	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

Member Length : 3' 7"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - HF	3.00"	3.00"	1.50"	1192	72	381	1573	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.50"	1192	72	381	1573	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 7" o/c	
Bottom Edge (Lu)	3' 7" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 3' 7"	N/A	13.0	--	--	
1 - Uniform (PSF)	0 to 3' 7" (Front)	1'	15.0	40.0	--	Default Load
2 - Uniform (PSF)	0 to 3' 7" (Front)	8' 6"	75.0	--	25.0	Default Load

• Side loads are assumed to not induce cross-grain tension.

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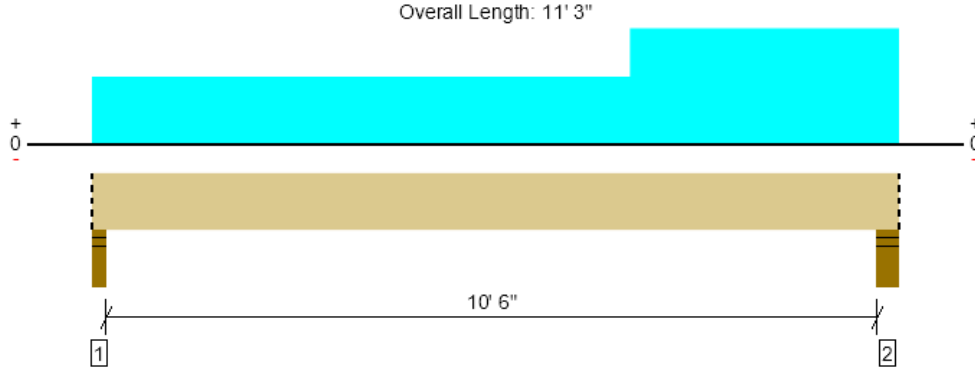
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Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



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 File Name: Nestler-Spare (2025)

Upper Floor Framing, Beam 2

1 piece(s) 3 1/2" x 11 7/8" 1.55E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1960 @ 2"	4961 (3.50")	Passed (40%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	1876 @ 9' 9 5/8"	9878	Passed (19%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	5557 @ 6'	18346	Passed (30%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.073 @ 5' 6 3/16"	0.269	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.174 @ 5' 7 11/16"	0.538	Passed (L/740)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

Member Length : 11' 3"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - HF	3.50"	3.50"	1.50"	1041	532	693	1960	Blocking
2 - Stud wall - HF	5.50"	5.50"	1.94"	1880	443	714	2747	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 3" o/c	
Bottom Edge (Lu)	11' 3" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 11' 3"	N/A	13.0	--	--	
1 - Uniform (PSF)	0 to 7' 6" (Front)	1'	30.0	60.0	--	Default Load
2 - Uniform (PSF)	0 to 7' 6" (Front)	1'	15.0	40.0	--	Default Load
3 - Uniform (PSF)	0 to 7' 6" (Front)	5'	20.0	--	25.0	Default Load
4 - Uniform (PSF)	7' 6" to 11' 3" (Front)	5'	75.0	--	25.0	Default Load
5 - Uniform (PSF)	7' 6" to 11' 3" (Front)	1'	75.0	60.0	--	Default Load

• Side loads are assumed to not induce cross-grain tension.

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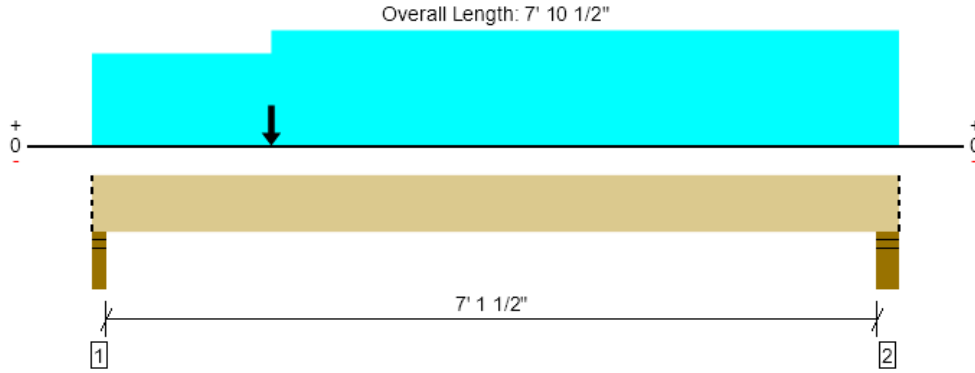
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Upper Floor Framing, Beam 3

1 piece(s) 3 1/2" x 11 7/8" 1.55E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4906 @ 2"	4961 (3.50")	Passed (99%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	3781 @ 3' 3 3/8"	9878	Passed (38%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	8403 @ 3' 7 11/16"	18346	Passed (46%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.046 @ 3' 9 7/16"	0.184	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.140 @ 3' 9 3/4"	0.369	Passed (L/633)	--	1.0 D + 1.0 S (All Spans)

Member Length : 7' 10 1/2"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - HF	3.50"	3.50"	3.46"	3267	154	1640	4906	Blocking
2 - Stud wall - HF	5.50"	5.50"	3.30"	3191	161	1482	4673	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 11" o/c	
Bottom Edge (Lu)	7' 11" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 7' 10 1/2"	N/A	13.0	--	--	
1 - Uniform (PSF)	0 to 7' 10 1/2" (Front)	1'	15.0	40.0	--	Default Load
2 - Uniform (PSF)	0 to 7' 10 1/2" (Front)	8' 6"	75.0	--	25.0	Default Load
3 - Uniform (PSF)	1' 9" to 7' 10 1/2" (Front)	5'	20.0	--	25.0	Default Load
4 - Point (lb)	1' 9" (Front)	N/A	604	--	683	Linked from: Beam 9, Support 2

• Side loads are assumed to not induce cross-grain tension.

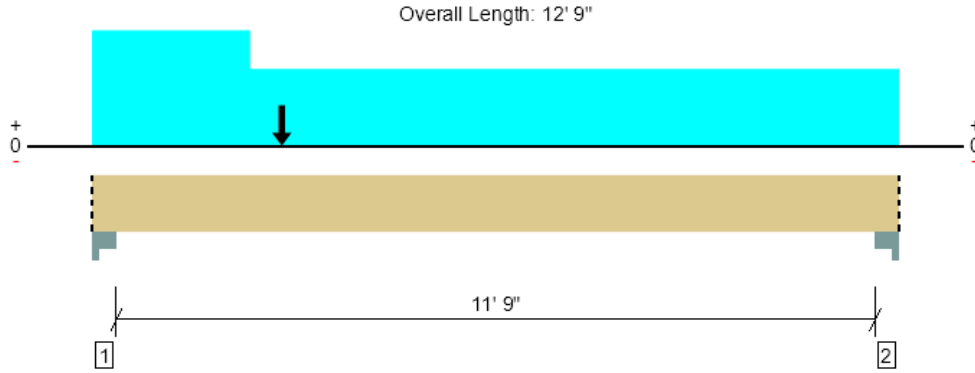
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Upper Floor Framing, Beam 4
1 piece(s) 5 1/4" x 14" 2.0E Parallam® PSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	14175 @ 4 1/2"	19688 (6.00")	Passed (72%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	9851 @ 1' 8"	14210	Passed (69%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	34157 @ 6' 1 3/16"	40743	Passed (84%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.294 @ 6' 4 7/16"	0.300	Passed (L/490)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.426 @ 6' 3 11/16"	0.600	Passed (L/338)	--	1.0 D + 1.0 L (All Spans)

Member Length : 12' 9"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Column Cap - steel	6.00"	6.00"	4.32"	5687	8488	1005	14175	Blocking
2 - Column Cap - steel	6.00"	6.00"	3.52"	3184	8353	281	11537	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	12' 9" o/c	
Bottom Edge (Lu)	12' 9" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 12' 9"	N/A	23.0	--	--	
1 - Uniform (PSF)	0 to 2' 6" (Front)	7' 2"	120.0	--	--	Default Load
2 - Uniform (PSF)	0 to 12' 9" (Front)	4' 3"	15.0	40.0	--	Default Load
3 - Uniform (PLF)	0 to 12' 9" (Front)	N/A	346.0	1132.0	--	Linked from: Joist C, Support 1
4 - Point (lb)	3' (Front)	N/A	1204	240	1286	Linked from: Beam 17, Support 2

• Side loads are assumed to not induce cross-grain tension.

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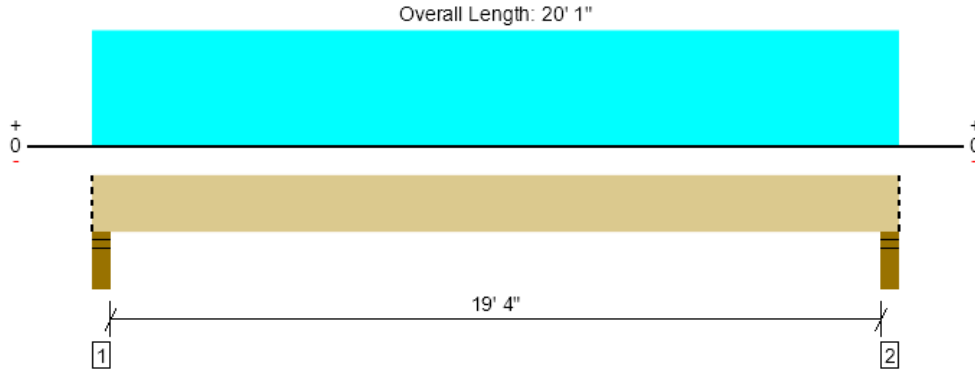
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Upper Floor Framing, Beam 6

1 piece(s) 3 1/2" x 11 7/8" 1.55E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	921 @ 3"	6379 (4.50")	Passed (14%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	796 @ 1' 4 3/8"	9878	Passed (8%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	4398 @ 10' 1/2"	18346	Passed (24%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.199 @ 10' 1/2"	0.490	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.417 @ 10' 1/2"	0.979	Passed (L/564)	--	1.0 D + 1.0 S (All Spans)

Member Length : 20' 1"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Stud wall - HF	4.50"	4.50"	1.50"	482	439	921	Blocking
2 - Stud wall - HF	4.50"	4.50"	1.50"	482	439	921	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	20' 1" o/c	
Bottom Edge (Lu)	20' 1" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 20' 1"	N/A	13.0	--	
1 - Uniform (PSF)	0 to 20' 1" (Front)	1' 9"	20.0	25.0	Default Load

• Side loads are assumed to not induce cross-grain tension.

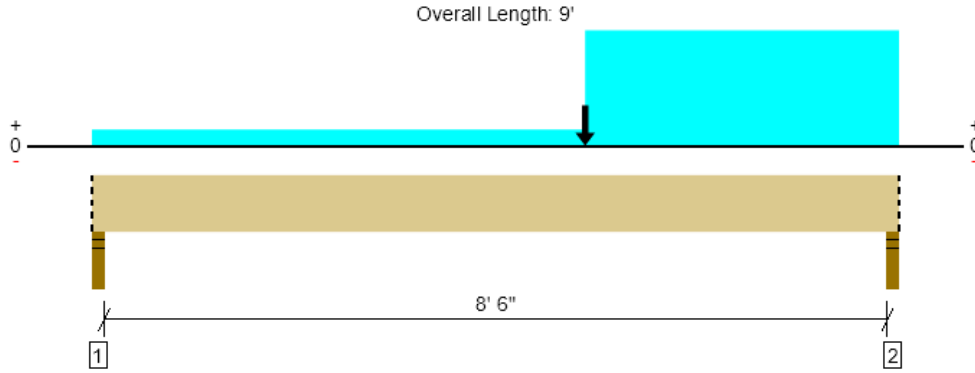
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Upper Floor Framing, Beam 17

1 piece(s) 3 1/2" x 11 7/8" 1.55E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2490 @ 8' 10 1/2"	4253 (3.00")	Passed (59%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	1891 @ 7' 9 1/8"	9878	Passed (19%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	5448 @ 5' 6"	18346	Passed (30%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.051 @ 5' 6"	0.219	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.100 @ 4' 9 3/8"	0.438	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

Member Length : 9'
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - HF	3.00"	3.00"	1.50"	583	240	523	1156	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.76"	1204	240	1286	2490	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	9' o/c	
Bottom Edge (Lu)	9' o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 9'	N/A	13.0	--	--	
1 - Uniform (PSF)	0 to 9' (Front)	1' 4"	15.0	40.0	--	Default Load
2 - Uniform (PSF)	5' 6" to 9' (Front)	10'	20.0	--	25.0	Default Load
3 - Point (lb)	5' 6" (Front)	N/A	790	--	934	Linked from: Beam 8, Support 1

• Side loads are assumed to not induce cross-grain tension.

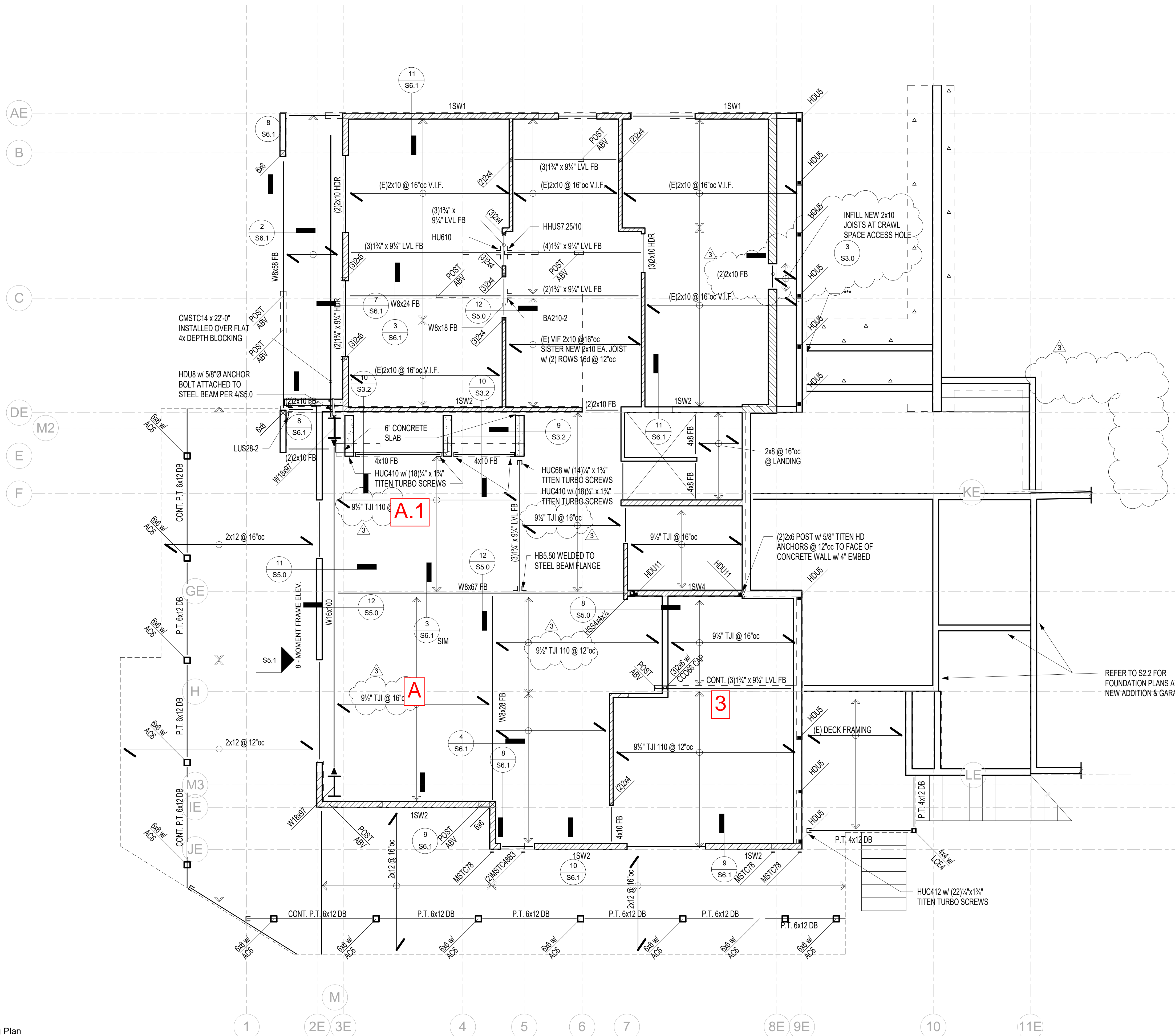
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FOUNDATION PLAN LEGEND

- ABRUPT CHANGE IN SLAB/FRAMING ELEVATION
- INDICATES EXISTING FOUNDATION
- INDICATES NEW FOUNDATION
- INDICATES DETAIL X ON SHEET SX.XX
- INDICATES SIMPSON HOLDOWN. REFER TO DETAIL 10/S3.0 & FOUNDATION PLAN NOTES FOR ANCHOR AND STUD STACK REQUIREMENTS.
- 2" PIPE PILE. REFER TO DETAILS ON SHEETS S3.0 & S3.1 FOR BATTERED PILES, WHERE APPLICABLE
- EPOXY EMBED (2#4x2'-0" BOT INTO (E) FOUNDATION 4" MIN USING SET-3G EPOXY

FOUNDATION PLAN NOTES

1. SLABS ON GRADE SHALL BE 4" THICK WITH 6x6 W1.4xW1.4 WWM CENTERED, U.N.O. PREPARED SOILS AND PROVIDE MINIMUM 10-MIL VISQUEEN VAPOR BARRIER UNDER ALL SLABS. REFER TO ARCHITECTURAL PLANS FOR SLAB ON GRADE ELEVATION AND TOPPING SLAB, WHERE OCCURS.
2. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS AND TOP OF SLAB ELEVATIONS.
3. ALL HOLDOWNS TO BE INSTALLED AS REQUIRED BY MANUFACTURER. REFER TO HOLDOWN SCHEDULE 10/S3.0.
4. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS AND NOTIFY ENGINEER OF ANY DISCREPANCIES.

FRAMING PLAN LEGEND

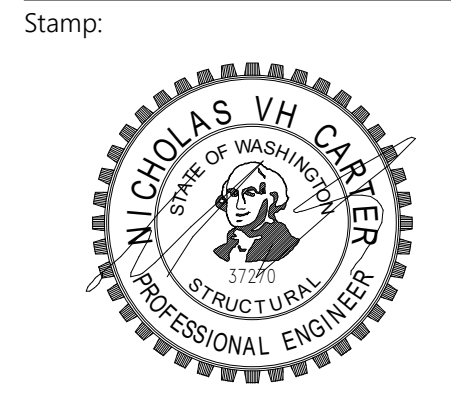
- WALLS BELOW
- COLUMNS BELOW
- HANGER
- ABRUPT CHANGE IN SLAB/FRAMING ELEVATION
- INDICATES DETAIL X ON SHEET SX.XX
- FRAMING SPAN AND EXTENTS
- DB INDICATES DROPPED BEAM
- FB INDICATES FLUSH BEAM
- TYP. UNO TYPICAL UNLESS NOTED OTHERWISE

FRAMING PLAN NOTES (TYPICAL UNLESS NOTED OTHERWISE)

1. FLOOR SHEATHING SHALL BE 23/32" TONGUE AND GROOVE APA RATED SHEATHING (SPAN RATING 40/20). NAIL AT ALL FRAMED PANEL EDGES AND OVER SHEAR WALLS w/ 10d @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING
2. ROOF SHEATHING SHALL BE 15/32" APA RATED SHEATHING (SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER SHEARWALLS w/ 8d @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING.
3. 3/4" T&G DECKING SHALL BE NAILED TO FRAMING BELOW WITH 10d @ 6"oc. MINIMUM (2) NAILS PER DECKING BOARD.
4. 1SWX INDICATES SHEAR WALL PER SCHEDULE 12/S6.0.
5. 2SWX INDICATES DOUBLE SIDED SHEAR WALL PER SCHEDULE 12/S6.0.
6. ALL HEADERS SHALL BE (2)2x8 U.N.O. REFER TO NOTE 5 FOR SUPPORT REQUIREMENTS.
7. COLUMNS SHALL BE DOUBLE STUDS MINIMUM, U.N.O., WITH BEAM OR HEADER BEARING FULLY ON COLUMN.
8. WHERE FULL HEIGHT LSL STUDS ARE CALLED OUT, INSTALL 1.3E 1 1/2" x 3 1/2" LSL STUDS @ 16"oc.

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Revision: Date:
MERCER ISLAND BUILDING PERMIT REV 1 03.28.25
MERCER ISLAND BUILDING PERMIT REV 2 06.10.25



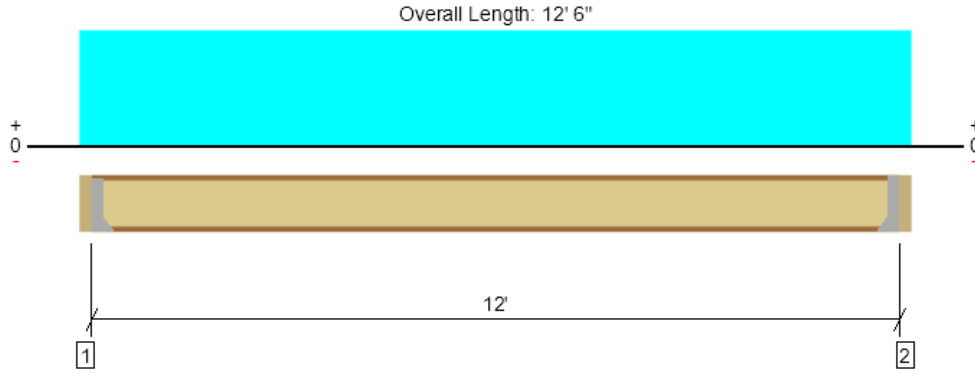
Consultants:

Project:
NS Residence
project no. 2401
8265 SE 61st St
Mercer Island, WA 98040

Drawing Title:
Main Floor Framing Plan

Date: June 9, 2025
Issued For: Permit Comments #2
Drawn By:
Checked By:
Scale: As indicated
Sheet No.:

Main Floor Framing, Joist A
1 piece(s) 9 1/2" TJI® 110 @ 16" OC



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	536 @ 3"	910 (1.75")	Passed (59%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	536 @ 3"	1220	Passed (44%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1608 @ 6' 3"	2500	Passed (64%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.145 @ 6' 3"	0.300	Passed (L/992)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.243 @ 6' 3"	0.600	Passed (L/592)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	49	45	Passed	--	--

Member Length : 12'
 System : Floor
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on 9 1/2" HF beam	3.00"	Hanger ¹	1.75" / - ²	225	333	558	See note ¹
2 - Hanger on 9 1/2" HF beam	3.00"	Hanger ¹	1.75" / - ²	225	333	558	See note ¹

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.
- ² Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 11" o/c	
Bottom Edge (Lu)	12' o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie							
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories	
1 - Face Mount Hanger	IUS1.81/9.5	2.00"	N/A	8-10dx1.5	2-Strong-Grip		
2 - Face Mount Hanger	IUS1.81/9.5	2.00"	N/A	8-10dx1.5	2-Strong-Grip		

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

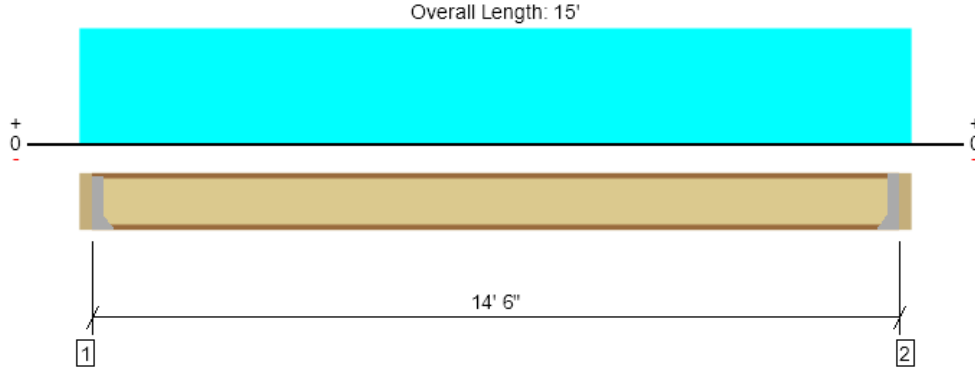
Vertical Load	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 12' 6"	16"	27.0	40.0	Default Load

ForTEWEB Software Operator	Job Notes
Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



7/28/2025 8:06:16 PM UTC
 ForteWEB v3.9, Engine: V8.4.3.94, Data: V8.1.7.3
 File Name: Nestler-Spare (2025)

Main Floor Framing, Joist A.1
1 piece(s) 9 1/2" TJI® 110 @ 12" OC



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	486 @ 3"	910 (1.75")	Passed (53%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	486 @ 3"	1220	Passed (40%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1761 @ 7' 6"	2500	Passed (70%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.229 @ 7' 6"	0.363	Passed (L/760)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.384 @ 7' 6"	0.725	Passed (L/454)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	45	45	Passed	--	--

Member Length : 14' 6"
 System : Floor
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on 9 1/2" HF beam	3.00"	Hanger ¹	1.75" / - ²	203	300	503	See note ¹
2 - Hanger on 9 1/2" HF beam	3.00"	Hanger ¹	1.75" / - ²	203	300	503	See note ¹

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.
- ² Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 8" o/c	
Bottom Edge (Lu)	14' 6" o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie							
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories	
1 - Face Mount Hanger	IUS1.81/9.5	2.00"	N/A	8-10dx1.5	2-Strong-Grip		
2 - Face Mount Hanger	IUS1.81/9.5	2.00"	N/A	8-10dx1.5	2-Strong-Grip		

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

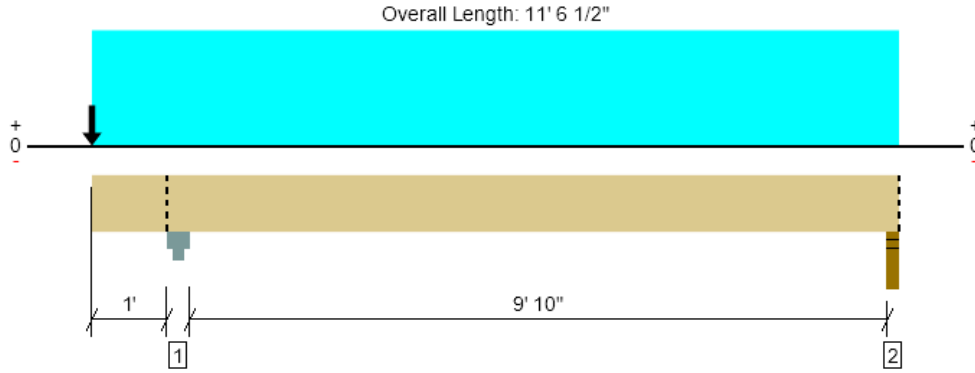
Vertical Load	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 15'	12"	27.0	40.0	Default Load

ForTEWEB Software Operator	Job Notes
Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



Main Floor Framing, Beam 3

3 piece(s) 1 3/4" x 9 1/4" 2.0E Microllam® LVL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	10211 @ 1' 2 3/4"	21656 (5.50")	Passed (47%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	8727 @ 2 3/4"	10611	Passed (82%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	-10760 @ 1' 2 3/4"	19327	Passed (56%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.081 @ 0	0.200	Passed (2L/364)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.142 @ 0	0.200	Passed (2L/208)	--	1.0 D + 1.0 S (All Spans)

Member Length : 11' 6 1/2"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2021
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Overhang deflection criteria: LL (0.2") and TL (0.2").
- Allowed moment does not reflect the adjustment for the beam stability factor.
- -690 lbs uplift at support located at 11' 5". Strapping or other restraint may be required.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Column Cap - steel	5.50"	5.50"	2.59"	4942	682	5269	10211	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.50"	-123	557/-8	-567	434/-690	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 7" o/c	
Bottom Edge (Lu)	11' 7" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 11' 6 1/2"	N/A	14.2	--	--	
1 - Uniform (PSF)	0 to 11' 6 1/2" (Front)	1' 4"	27.0	40.0	--	Default Load
2 - Uniform (PSF)	0 to 11' 6 1/2" (Front)	1' 4"	15.0	40.0	--	Default Load
3 - Point (lb)	0 (Front)	N/A	4009	--	4702	Linked from: Beam 16, Support 1

• Side loads are assumed to not induce cross-grain tension.

Weyerhaeuser Notes

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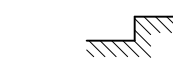
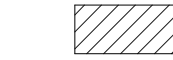






The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Steven Nickolas Carter Quinn Norlin (206) 264-7784 ssn@cqn-se.com	



7/28/2025 8:06:16 PM UTC
 ForteWEB v3.9, Engine: V8.4.3.94, Data: V8.1.7.3
 File Name: Nestler-Spare (2025)

FOUNDATION PLAN LEGEND

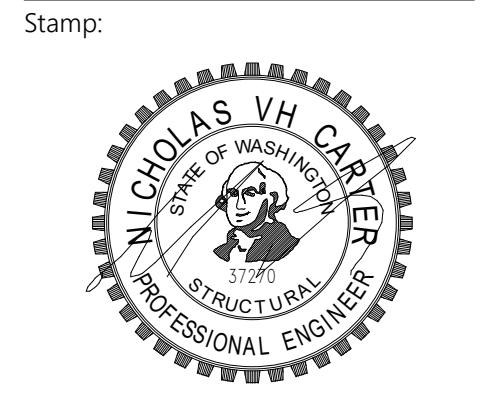
-  ABRUPT CHANGE IN SLAB/FRAMING ELEVATION
-  INDICATES EXISTING FOUNDATION
-  INDICATES NEW FOUNDATION
-  INDICATES DETAIL X ON SHEET SX.XX
-  INDICATES SIMPSON HOLDOWN. REFER TO DETAIL 10/S3.0 & FOUNDATION PLAN NOTES FOR ANCHOR AND STUD STACK REQUIREMENTS.
-  2"Ø PIPE PILE. REFER TO DETAILS ON SHEETS S3.0 & S3.1 FOR BATTERED PILES, WHERE APPLICABLE
-  3"Ø PIPE PILE. REFER TO DETAILS ON SHEETS S3.0 & S3.1 FOR BATTERED PILES, WHERE APPLICABLE
-  EPOXY EMBED (2#4x2'-0" BOT INTO (E) FOUNDATION 4" MIN USING SET-3G EPOXY

FOUNDATION PLAN NOTES

1. STRUCTURAL SLAB ON GRADE AND REINFORCING PER PLAN. PREPARE SOILS AND PROVIDE MINIMUM 10-MIL VISQUEEN VAPOR BARRIER UNDER ALL SLABS. REFER TO ARCHITECTURAL PLANS FOR SLAB ON GRADE ELEVATION AND TOPPING SLAB, WHERE OCCURS.
2. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS AND TOP OF SLAB ELEVATIONS.
3. ALL HOLDOWNS TO BE INSTALLED AS REQUIRED BY MANUFACTURER. REFER TO HOLDOWN SCHEDULE 10/S3.0.
4. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS AND NOTIFY ENGINEER OF ANY DISCREPANCIES.

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Revision:	Date:
⚠ MERCER ISLAND BUILDING PERMIT REV 1	03.28.25
⚠ MERCER ISLAND BUILDING PERMIT REV 2	06.10.25
⚠ MERCER ISLAND BUILDING PERMIT REV 3	07.28.25



Consultants:

Project:
NS Residence
project no. 2401
8265 SE 61st St
Mercer Island, WA 98040

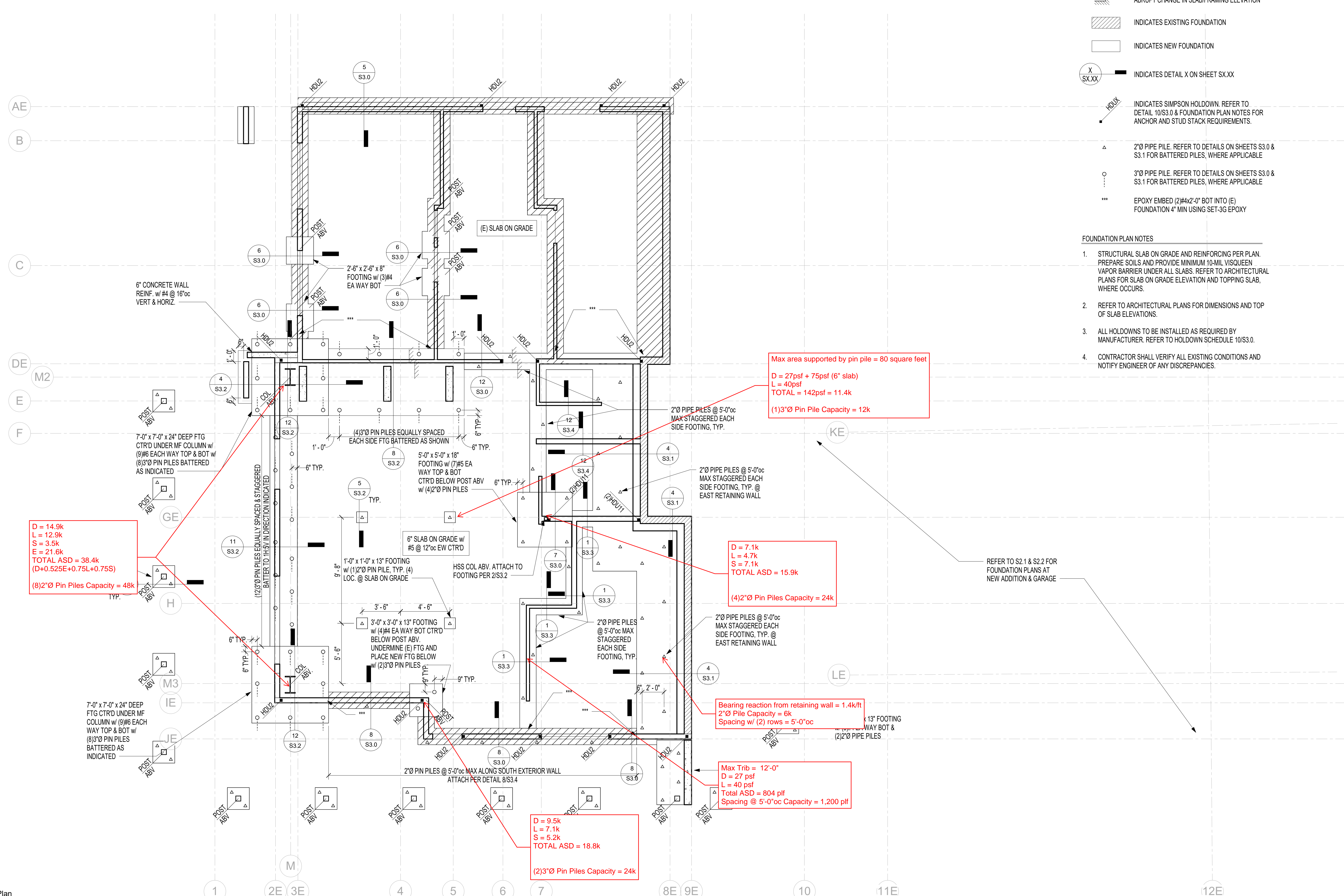
Drawing Title:
Foundation Plan

Date: June 9, 2025
Issued For: Permit Comments #2

Drawn By:
Checked By:
Scale: As indicated

Sheet No.:

S2.0



D = 14.9k
L = 12.9k
S = 3.5k
E = 21.6k
TOTAL ASD = 38.4k
(D+0.525E+0.75L+0.75S)
(8)2"Ø Pin Piles Capacity = 48k

Max area supported by pin pile = 80 square feet
D = 27psf + 75psf (6" slab)
L = 40psf
TOTAL = 142psf = 11.4k
(1)3"Ø Pin Pile Capacity = 12k

D = 7.1k
L = 4.7k
S = 7.1k
TOTAL ASD = 15.9k
(4)2"Ø Pin Piles Capacity = 24k

Bearing reaction from retaining wall = 1.4k/ft
2"Ø Pile Capacity = 6k
Spacing w/ (2) rows = 5'-0"oc

Max Trib = 12'-0"
D = 27 psf
L = 40 psf
Total ASD = 804 plf
Spacing @ 5'-0"oc Capacity = 1,200 plf

D = 9.5k
L = 7.1k
S = 5.2k
TOTAL ASD = 18.8k
(2)3"Ø Pin Piles Capacity = 24k

1 Foundation Plan
1/4" = 1'-0"

Concrete Beam

Project File: Nestler-Spare (2025).ec6

LIC# : KW-06015393, Build:20.24.10.30

BYKONEN CARTER QUINN

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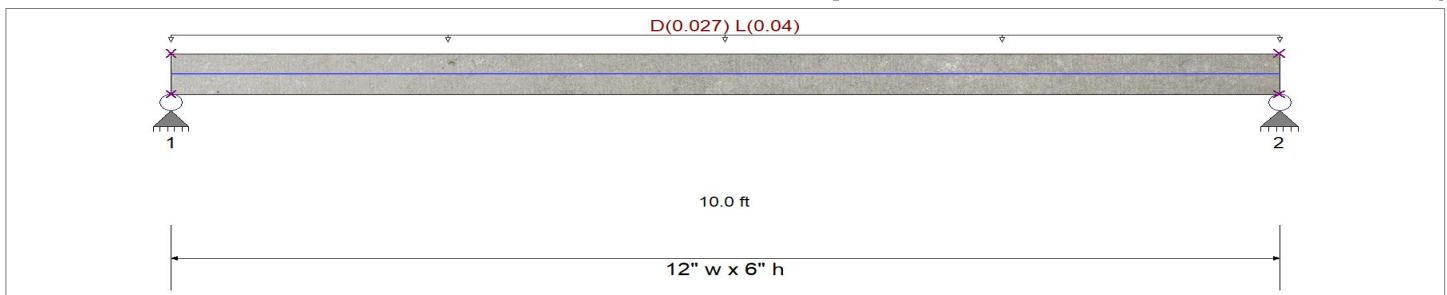
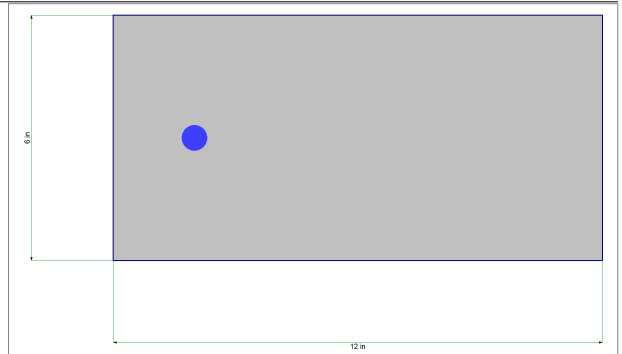
DESCRIPTION: Structural Slab on Grade

CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019
 Load Combination Set : ASCE 7-16

General Information

f'_c	=	2.50 ksi	ϕ Phi Values	Flexure :	0.90
$f_r = 7.5 * \lambda * f'_c / 2$	=	375.0 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.850
λ LtWt Factor	=	1.0			
Elastic Modulus	=	2,850.0 ksi	Fy - Stirrups	=	40.0 ksi
fy - Main Rebar	=	60.0	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #	=	3
			Number of Resisting Legs Per Stirrup	=	2



Cross Section & Reinforcing Details

Rectangular Section, Width = 12.0 in, Height = 6.0 in
 Span #1 Reinforcing....
 1-#5 at 3.0 in from Bottom, from 0.0 to 10.0 ft in this span

Beam self weight calculated and added to loads
Load for Span Number 1

Uniform Load : D = 0.0270, L = 0.040 k/ft, Tributary Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.624	: 1
Section used for this span		Typical Section	
Mu : Applied		2.292	k-ft
Mn * Phi : Allowable		3.676	k-ft
Location of maximum on span		5.009	ft
Span # where maximum occurs		Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.015 in	Ratio =	8210	>=480.0	L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<480.0	L Only
Max Downward Total Deflection	0.051 in	Ratio =	2354	>=240.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<240.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	0.697	0.697
Max Upward from Load Combinations	0.697	0.697
Max Upward from Load Cases	0.497	0.497
D Only	0.497	0.497
+D+L	0.697	0.697
+D+0.750L	0.647	0.647
+0.60D	0.298	0.298

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Concrete Beam

Project File: Nestler-Spare (2025).ec6

LIC# : KW-06015393, Build:20.24.10.30

BYKONEN CARTER QUINN

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DESCRIPTION: Structural Slab on Grade

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
L Only	0.200	0.200

Shear Stirrup Requirements

Entire Beam Span Length : $V_u < \Phi \cdot V_c / 2$, Req'd Vs = Not Req'd per 9.6.3.1, Stirrups are not required.

Detailed Shear Information

Load Combination	Span Number	Distance 'd'		Vu (k)		Mu (k-ft)	d*Vu/Mu	Phi*Vc (k)	Comment	Phi*Vs (k)	Phi*Vn (k)	Spacing (in) Req'd
		(ft)	(in)	Actual	Design							
+1.20D+1.60L	1	0.00	3.00	0.92	0.92	0.00	1.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.11	3.00	0.90	0.90	0.10	1.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.22	3.00	0.88	0.88	0.20	1.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.33	3.00	0.86	0.86	0.29	0.74	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.44	3.00	0.84	0.84	0.38	0.55	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.55	3.00	0.82	0.82	0.47	0.43	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.66	3.00	0.80	0.80	0.56	0.35	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.77	3.00	0.78	0.78	0.65	0.30	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.87	3.00	0.76	0.76	0.73	0.26	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	0.98	3.00	0.74	0.74	0.81	0.23	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.09	3.00	0.72	0.72	0.89	0.20	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.20	3.00	0.70	0.70	0.97	0.18	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.31	3.00	0.68	0.68	1.04	0.16	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.42	3.00	0.66	0.66	1.12	0.15	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.53	3.00	0.64	0.64	1.19	0.13	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.64	3.00	0.62	0.62	1.26	0.12	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.75	3.00	0.60	0.60	1.32	0.11	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.86	3.00	0.58	0.58	1.39	0.10	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	1.97	3.00	0.56	0.56	1.45	0.10	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.08	3.00	0.54	0.54	1.51	0.09	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.19	3.00	0.52	0.52	1.57	0.08	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.30	3.00	0.50	0.50	1.62	0.08	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.40	3.00	0.48	0.48	1.67	0.07	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.51	3.00	0.46	0.46	1.73	0.07	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.62	3.00	0.44	0.44	1.77	0.06	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.73	3.00	0.42	0.42	1.82	0.06	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.84	3.00	0.40	0.40	1.87	0.05	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	2.95	3.00	0.38	0.38	1.91	0.05	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.06	3.00	0.36	0.36	1.95	0.05	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.17	3.00	0.34	0.34	1.99	0.04	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.28	3.00	0.32	0.32	2.02	0.04	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.39	3.00	0.30	0.30	2.05	0.04	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.50	3.00	0.28	0.28	2.09	0.03	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.61	3.00	0.26	0.26	2.11	0.03	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.72	3.00	0.24	0.24	2.14	0.03	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.83	3.00	0.22	0.22	2.17	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	3.93	3.00	0.20	0.20	2.19	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.04	3.00	0.18	0.18	2.21	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.15	3.00	0.16	0.16	2.23	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.26	3.00	0.14	0.14	2.24	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.37	3.00	0.12	0.12	2.26	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.48	3.00	0.10	0.10	2.27	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.59	3.00	0.08	0.08	2.28	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.70	3.00	0.06	0.06	2.28	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.81	3.00	0.04	0.04	2.29	0.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	4.92	3.00	0.02	0.02	2.29	0.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	5.03	3.00	-0.01	0.01	2.29	0.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	5.14	3.00	-0.03	0.03	2.29	0.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	5.25	3.00	-0.05	0.05	2.29	0.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	5.36	3.00	-0.07	0.07	2.28	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	5.46	3.00	-0.09	0.09	2.27	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0
+1.20D+1.60L	1	5.57	3.00	-0.11	0.11	2.26	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	0.0

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Concrete Beam

Project File: Nestler-Spare (2025).ec6

LIC# : KW-06015393, Build:20.24.10.30

BYKONEN CARTER QUINN

(c) ENERCALC, LLC 1982-2025

DESCRIPTION: Structural Slab on Grade

Detailed Shear Information

Load Combination	Span Number	Distance 'd'		Vu (k)		Mu (k-ft)	d*Vu/Mu	Phi*Vc (k)	Comment	Phi*Vs (k)	Phi*Vn (k)	Spacing (in) Req'd
		(ft)	(in)	Actual	Design							
+1.20D+1.60L	1	5.68	3.00	-0.13	0.13	2.25	0.01	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	5.79	3.00	-0.15	0.15	2.23	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	5.90	3.00	-0.17	0.17	2.22	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.01	3.00	-0.19	0.19	2.20	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.12	3.00	-0.21	0.21	2.18	0.02	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.23	3.00	-0.23	0.23	2.15	0.03	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.34	3.00	-0.25	0.25	2.13	0.03	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.45	3.00	-0.27	0.27	2.10	0.03	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.56	3.00	-0.29	0.29	2.07	0.03	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.67	3.00	-0.31	0.31	2.04	0.04	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.78	3.00	-0.33	0.33	2.00	0.04	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.89	3.00	-0.35	0.35	1.97	0.04	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	6.99	3.00	-0.37	0.37	1.93	0.05	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.10	3.00	-0.39	0.39	1.89	0.05	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.21	3.00	-0.41	0.41	1.84	0.06	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.32	3.00	-0.43	0.43	1.80	0.06	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.43	3.00	-0.45	0.45	1.75	0.06	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.54	3.00	-0.47	0.47	1.70	0.07	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.65	3.00	-0.49	0.49	1.65	0.07	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.76	3.00	-0.51	0.51	1.59	0.08	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.87	3.00	-0.53	0.53	1.54	0.09	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	7.98	3.00	-0.55	0.55	1.48	0.09	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.09	3.00	-0.57	0.57	1.42	0.10	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.20	3.00	-0.59	0.59	1.36	0.11	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.31	3.00	-0.61	0.61	1.29	0.12	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.42	3.00	-0.63	0.63	1.22	0.13	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.52	3.00	-0.65	0.65	1.15	0.14	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.63	3.00	-0.67	0.67	1.08	0.15	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.74	3.00	-0.69	0.69	1.01	0.17	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.85	3.00	-0.71	0.71	0.93	0.19	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	8.96	3.00	-0.73	0.73	0.85	0.21	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.07	3.00	-0.75	0.75	0.77	0.24	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.18	3.00	-0.77	0.77	0.69	0.28	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.29	3.00	-0.79	0.79	0.61	0.33	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.40	3.00	-0.81	0.81	0.52	0.39	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.51	3.00	-0.83	0.83	0.43	0.48	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.62	3.00	-0.85	0.85	0.34	0.63	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.73	3.00	-0.87	0.87	0.24	0.89	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.84	3.00	-0.89	0.89	0.15	1.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	
+1.20D+1.60L	1	9.95	3.00	-0.91	0.91	0.05	1.00	2.70	Vu < Phi*Vc / 2	2.7	0.0	

Maximum Forces & Stresses for Load Combinations

Load Combination Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft)		
			Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	10.000	2.29	3.68	0.62
+1.40D					
Span # 1	1	10.000	1.74	3.68	0.47
+1.20D+1.60L					
Span # 1	1	10.000	2.29	3.68	0.62
+1.20D+L					
Span # 1	1	10.000	1.99	3.68	0.54
+1.20D					
Span # 1	1	10.000	1.49	3.68	0.41
+0.90D					
Span # 1	1	10.000	1.12	3.68	0.30

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft)	Load Combination	Max. "+" Defl (in)	Location in Span (ft)
+D+L	1	0.0510	5.000		0.0000	0.000